Application and Validation of Simulated BBP & Cybershake Motions for Building Response Analyses

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Background

Broad objective: Test and validate the use of simulated ground motions from SCEC's Broadband Platform (BBP) & Cybershake in nonlinear response history analysis and collapse assessment of tall buildings.

Scope: For two archetype buildings at three sites,

- 1. Select hazard-consistent ground motions for target hazard parameters (Sa response spectra intensity, shape and duration)
- 2. Analyzed structural responses under recorded and simulated motions, which are selected/scaled to the same target spectra
- 3. Contrast and evaluate building demand parameters
- 4. Identify the benefits and limitations of using simulated earthquake seismograms in engineering applications.

Archetype Buildings

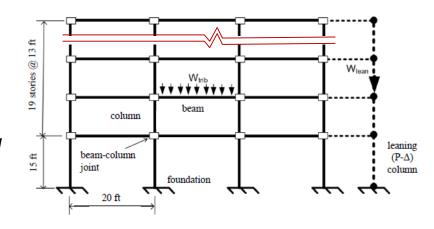
20-story RC frame

- **Archetype ID:** 1020 (FEMA P695)
- Modeling: Cyclic hinge model for beams and columns in the lateral frame + leaning system (gravity frame)
- Period: $T_1 = 2.6$ sec, T2 = 0.9 sec

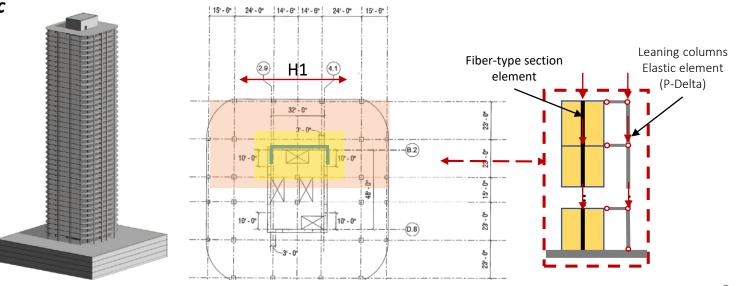
42-story RC frame

- **Archetype ID:** simplified from TBI building 1C (H1 direction)
- Modeling: force-based fiber element for shear wall + leaning system (gravity frame)

• Period: $T_1 = 4.2 \text{ sec}$, $T_2 = 1.0 \text{ sec}$

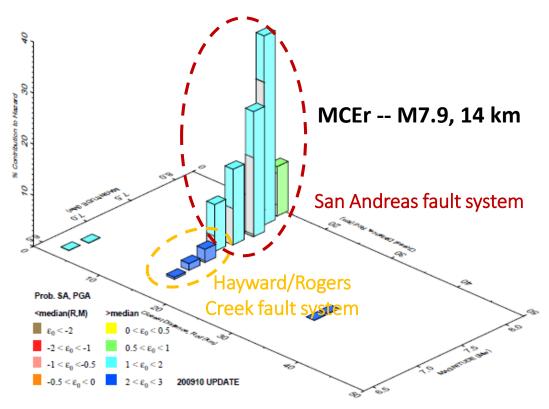


2D OpenSEES model: 20-story RC frame



San Francisco Downtown, SFDT (8029-RIN)

[1728 BBP Records]



Major seismic sources for SFDT site

Key Focus Areas

GM selection

- 1. Single-target IMs (Conditional Spectra + duration)
- 2. ASCE-7 MCEr procedure
- 3. Simulated BBP is more consistent with governing hazard (requiring less scaling)

Structural response & collapse

- 1. Ability to match targets
- 2. Sensitivity of response to GMs
- 3. Contrasting CS versus UHS

Intensity Measures (IMs)

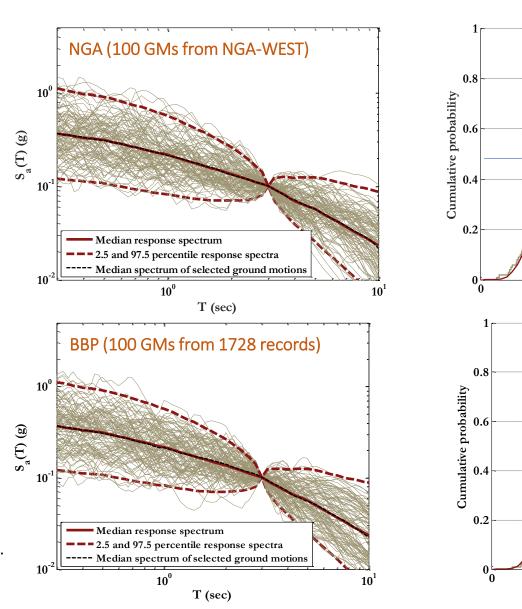
- Target: Conditional Spectra (CS)^[1] and significant duration, Ds_{5-75} ^[2]
- Prediction equations for spectral acceleration, Sa^[3] and duration^[4] with correlations to construct Generalized Conditional Intensity Measure (GCIM)^[5]: CS + Ds₅₋₇₅

Ground motion selection

- Expanded selection algorithm^[6]
 with different weights to CS and
 duration^[7] to optimize selection
- Results (10%/50yr & 2%/50yr):
 Recorded NGA and simulated BBP motions match target IMs well

Ref:

- [1]: Lin, Harmsen, Baker, & Luco (2013).
- [2]: Chandramohan, Baker, & Deierlein (2016).
- [3]: Campbell & Bozorgnia (2008).
- [4]: Afshari & Stewart (2016).
- [5]: Bradley (2010).
- [6]: Jayaram, Lin, & Baker (2011).
- [7]: Bijelic, Lin, & Deierlein.



Ground motion selection ($T_{cond} = 3 \text{ s, SFDT, } 10\%/50\text{yr}$)

Median Ds₅₋₇₅=15sec

D_{\$5-75} (sec)

10

20

Significant duration

Significant duration

Emperical cumulative probability function

Target distribution (median = 14.6314 sec)

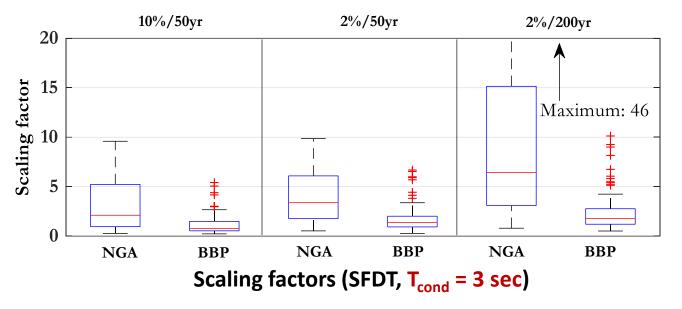
D_{\$5-75} (sec)

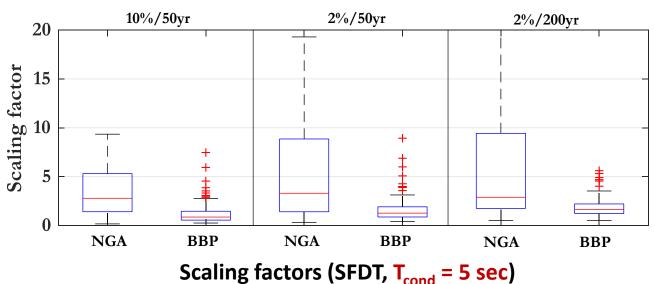
Emperical cumulative probability function

Target distribution (median = 14.631 sec)

Scaling factor

- Since BBP (v17.3, Part C) records are simulated from large events, they generally need much smaller scaling factors compared to NGA records
- The benefit of BBP (and simulations such as CyberShake) to avoid large scaling factors is apparent at high intensity levels
- Constrained by the number of simulation runs

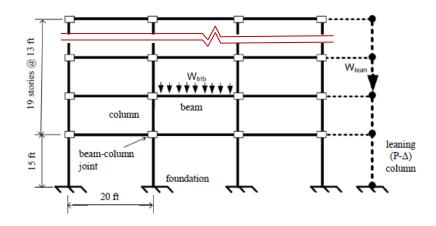


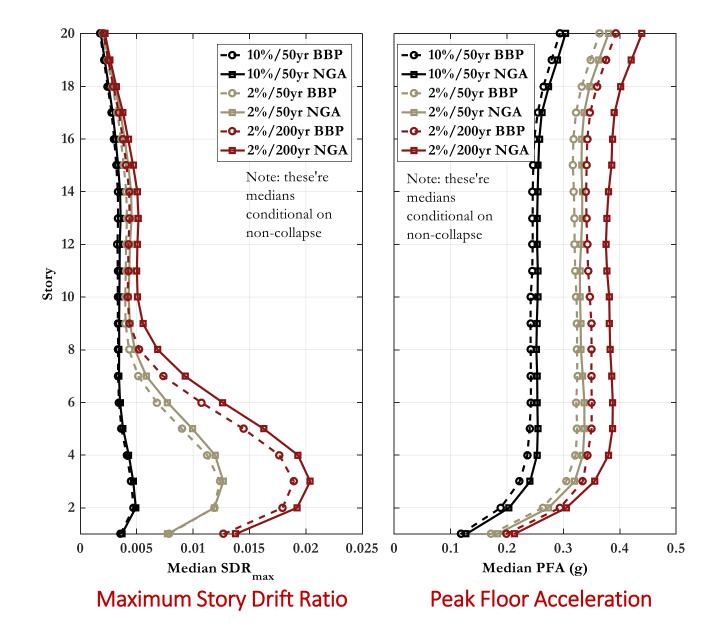


Multi-Stripe Analysis (MSA)

20-Story Frame Response

- Maximum Story Drift Ratio (SDR_{max})
 - Comparable up to 2%/50yr
 - <10% difference under 2%/200yr</p>
- Peak Floor Acceleration (PFA)
 - Comparable up to 2%/50yr
 - ~10% difference under 2%/200yr



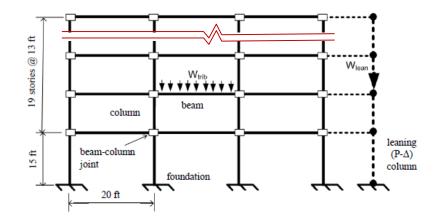


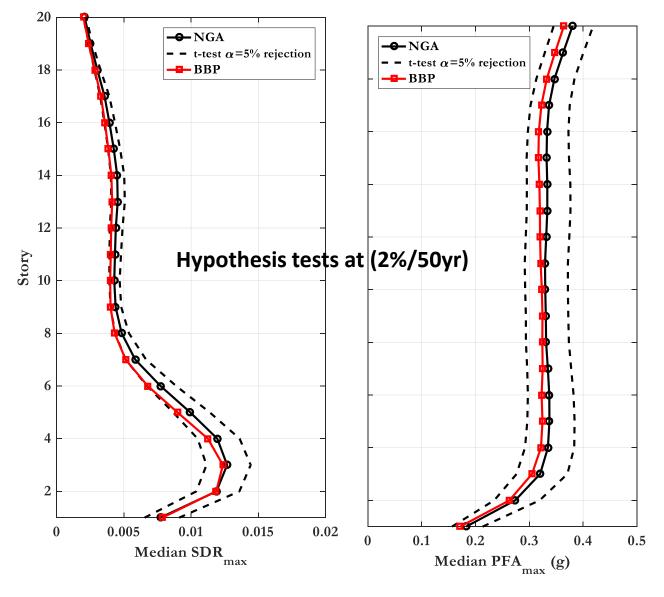
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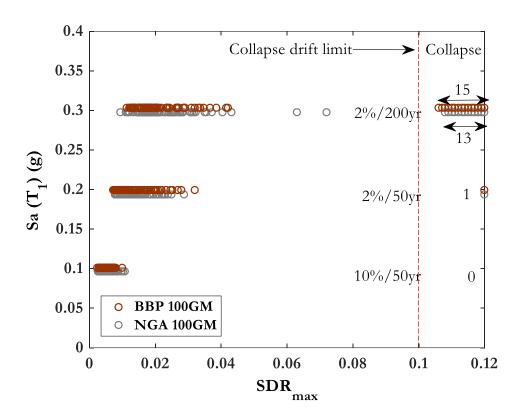


Maximum Story Drift Ratio

Peak Floor Acceleration

Collapse risk (20-story RC frame)

- NGA and BBP motions yield the same number of collapses at 10%/50yr and 2%/50yr levels
- Collapse (2%/200yr): BBP 15 vs. 13 NGA
- About 7% difference in median collapse intensity and about 15% difference in mean annual frequency (MAF) of collapse



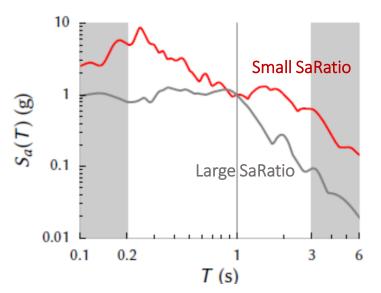
0.08 NGA: $Sa_{\rm m} = 0.45 \text{ g}, \lambda_{\rm col} = 5.6e^{-4}$ BBP: $Sa_m = 0.42 \text{ g}, \lambda_{col} = 6.4e^{-4}$ 0.4 0.06 Probability of collapse Seismic **BBP** 0.04 hazard curve NGA 0.02 0.1 0.1 0.2 0.3 0.4 0 0.5 Sa (g)

Stripe Analysis - Story Drift Ratios

Collapse Fragilities

Collapse deaggregation (20-story RC frame)

Although both selected GM sets fit the target Conditional Spectra, the BBP set has more records with smaller SaRatio's. This contributes to a slightly higher estimated collapse probability at 2%/200yr.

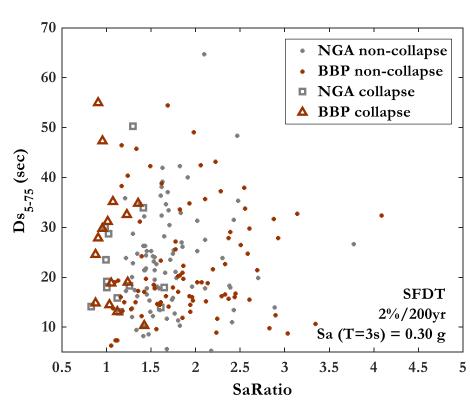


$$Sa, avg(T_0, T_2) = \exp\left[\frac{\int_{T_0}^{T_2} \ln Sa(\tau) d\tau}{T_2 - T_0}\right]$$

$$SaRatio(T_0, T_1, T_2) = \frac{Sa(T_1)}{Sa, avg(T_0, T_2)}$$

Sa, average & SaRatio

- Sa,avg: average PSA values over a period range
- SaRatio: the ratio between $Sa(T_1)$ and Sa, avg, which describes the period-dependent spectral shape



Collapse deaggregation (100 GMs)

Multi-Stripe Analysis (MSA)

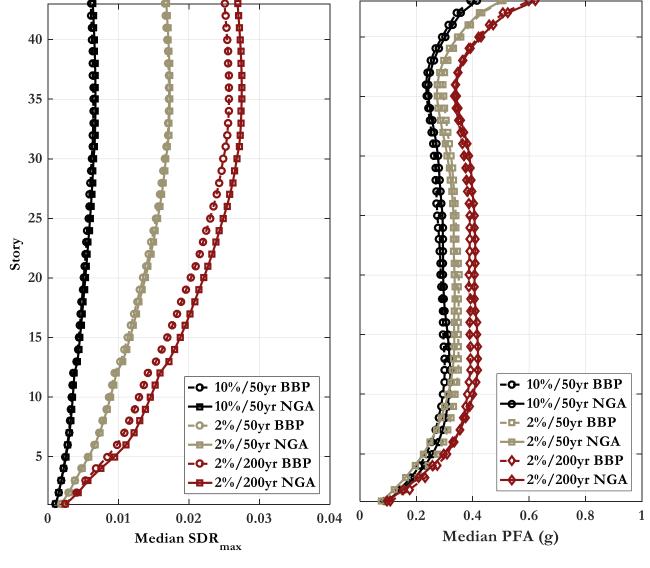
42-story shearwall

- Maximum Story Drift Ratio (SDR_{max})

 Comparable up to 2%/50yr
 difference under 2%/200yr
- Peak Floor Acceleration (PFA)

 Comparable up to 2%/50yr
 <10% difference under 2%/200yr





Maximum Story Drift Ratio

Peak Floor Acceleration

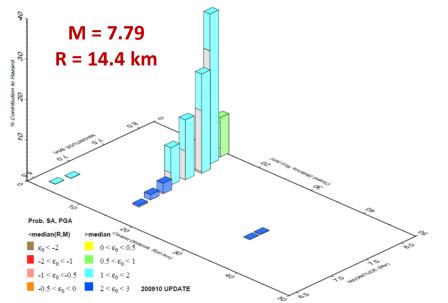
ASCE 7-16 Procedure

Ground motion selection

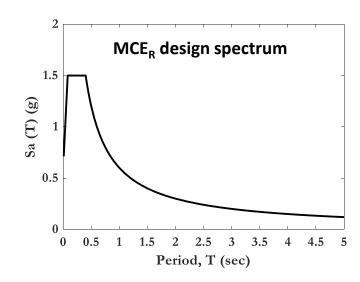
- Site information:
 - Site class: B
 - Ss = 1.5 q
 - S1 = 0.6 g
- Target: ASCE 7-16 MCE_R/DBE spectrum
- Record number: 11
- Criteria:
 - Consistent M, R, and mechanism
 - Close site condition
 - Mean Sa \geq target over $(0.2T_1 \sim 1.5 T_1)$

Oakland Sal Francisco Alameda* USGS design tool) Daly City

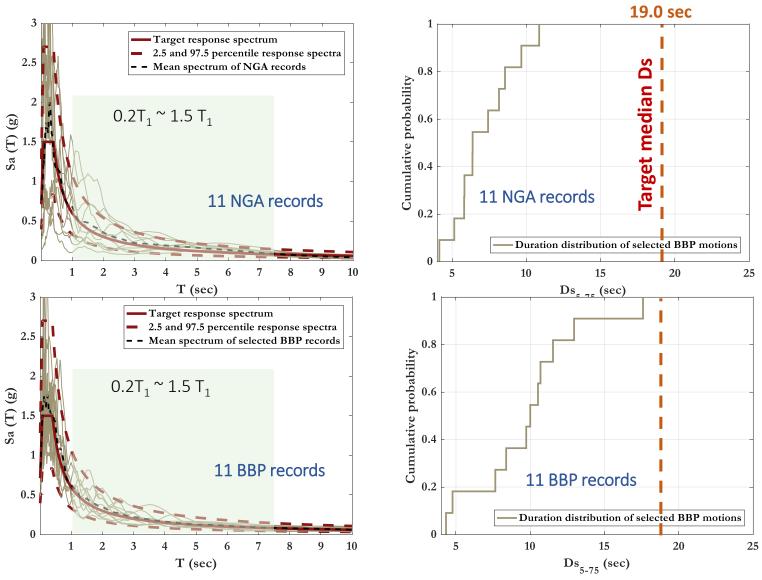
MCE_R and design response spectrum from ASCE-7 (after

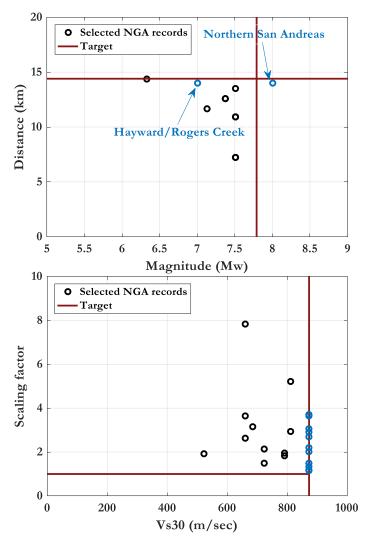


Seismic hazard deaggregation at 2% in 50 years intensity for 8029-RIN (after USGS)



MCEr, T = 5 sec: selected NGA vs. BBP records



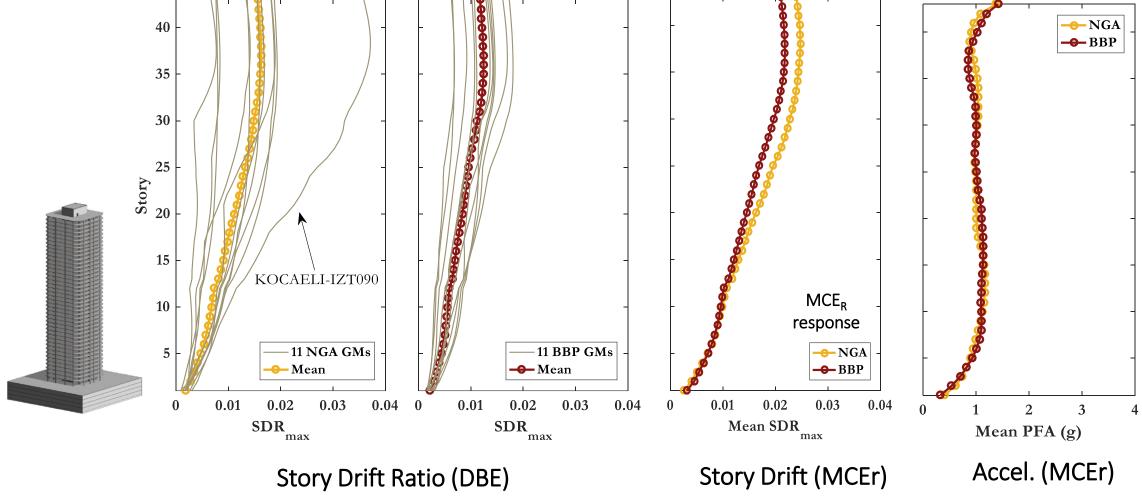


Causal comparison (M, R, Vs30, and scaling)

Comparison between selected 11 NGA and BBP records (Sa and duration)

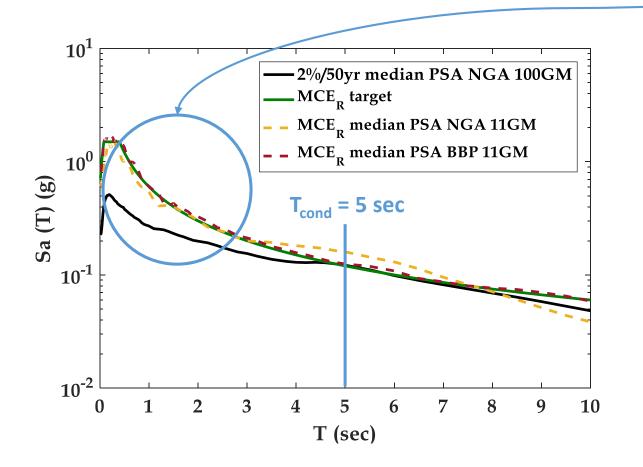
42-Story Shear Wall Response

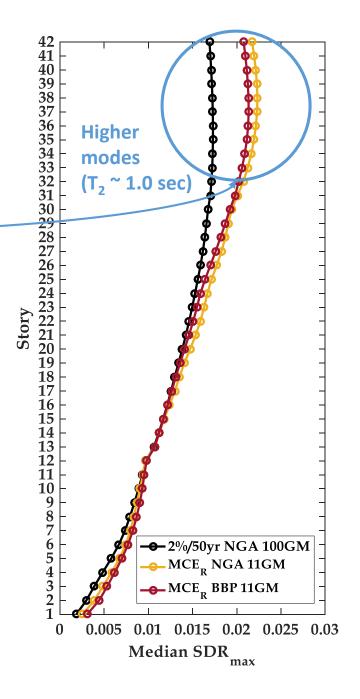
- Effects from longer duration is insignificant (drift is modest)
- NGA: drifts ~10% larger
- BBP: smaller variation in response



MCE_R (UHS) vs. 2%/50yr CS Target

- Median PSA of 11 GM under MCER is generally higher than the median PSA of 100 GM under 2%/50yr, especially in the high-frequency range
- Median responses under 11 GM are slightly larger than the median responses under 100 GM

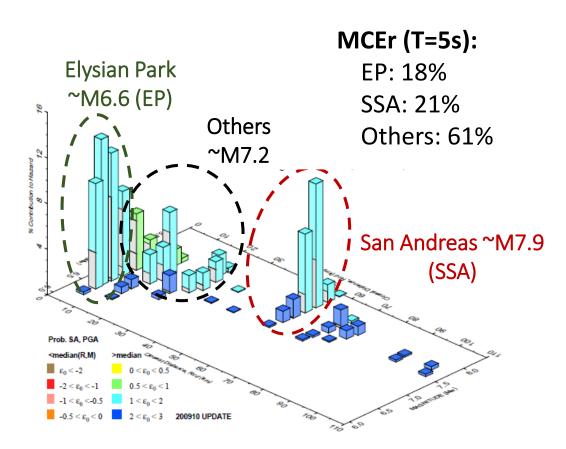




Summary (San Francisco Site)

- 1. Simulated (BBP) motions can be selected/scaled to match the PSHA target IMs (CS+D $_{s5-75}$) with much smaller scaling than existing recorded motions.
- 2. When ground motions are selected/scaled to comparable target IMs, there are no statistically significant differences in *calculated structural responses and collapse* risks between recorded and simulated motions.
- 3. Strictly following ASCE-7 guidelines (matching magnitude, mechanism and soil conditions), BBP simulations:
 - a) provide a better fit to the response spectra target (e.g., MCE_R) with *less* dispersion and closer match to expected durations,
 - b) can avoid "unrealistic" spectral shapes in selected records, thus offering more reliable response prediction from a limited number of ground motions.

Los Angeles Downtown (LADT)



Major seismic sources for LADT site

Key Focus Areas

GM selection

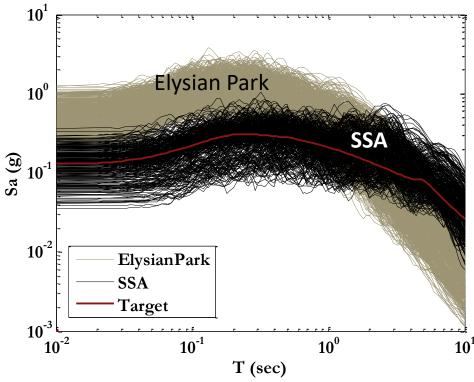
- 1. Multi-target IM's
- 2. Spectral shape of Elysian Park records

Structural response & collapse

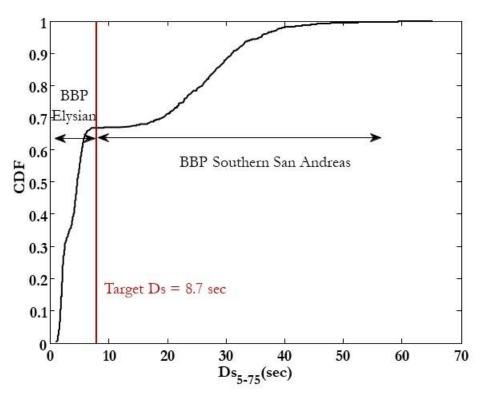
- 1. Investigate sensitivity to different GMPE
- 2. Investigate sensitivity to records with different causal parameters

Multi-target selection (LADT)

- 1. Competing seismic sources: (1) South San Andreas, (2) Elysian Park, and (3) others (e.g., Puente Hills) whose **mechanisms and GM characteristics are very different**
- 2. Select ground motions **for each major source target** based on their relative contributions to the total hazard (dependent on T_1 and return period)



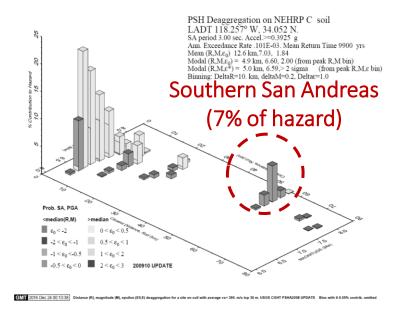
Target Spectrum vs. EP & SSA simulations (LADT, 10%/50yr, $T_{cond} = 5$ sec)



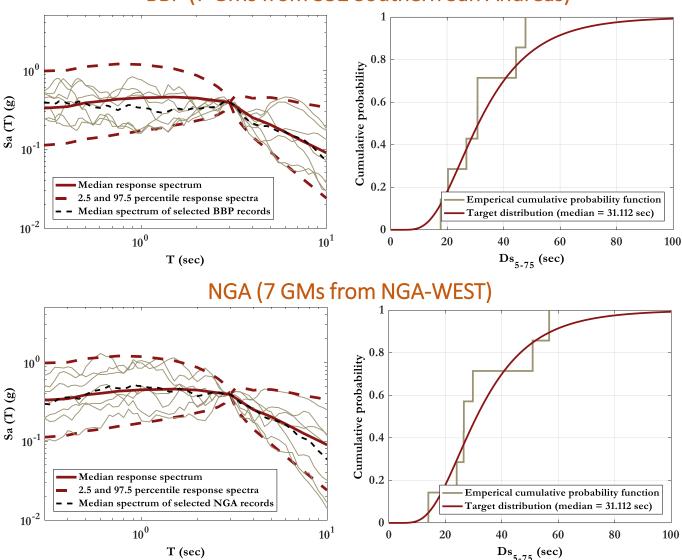
Target Duration vs. EP & SSA simulations (LADT, 10%/50yr, $T_{cond} = 5$ sec)

Southern San Andreas (T=3s, 2%/200yr)

Reasonable match to the targets for both BBP and NGA motions at all 3 intensities.



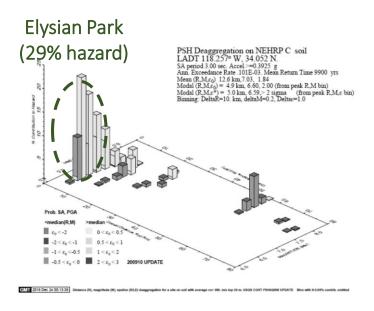
BBP (7 GMs from 352 Southern San Andreas)



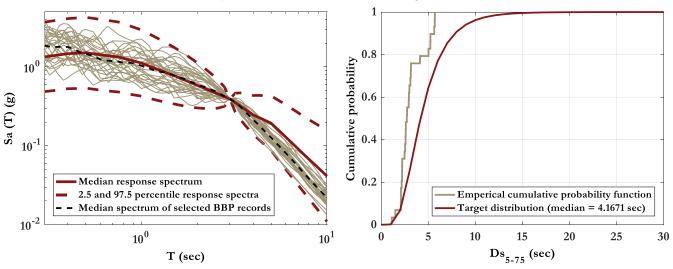
Ground motion selection ($T_{cond} = 3 \text{ s, LADT, } 2\% \text{ in } 200 \text{ years}$)

Elysian Park (T=3s, 2%/200yr)

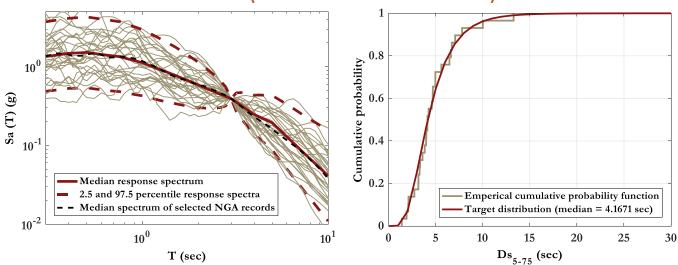
- Good agreement between selected records and target up to 2%/50yr level
- Discrepancy in BBP motions for 2%/200yr hazard



BBP (29 GMs from 704 Elysian Park)



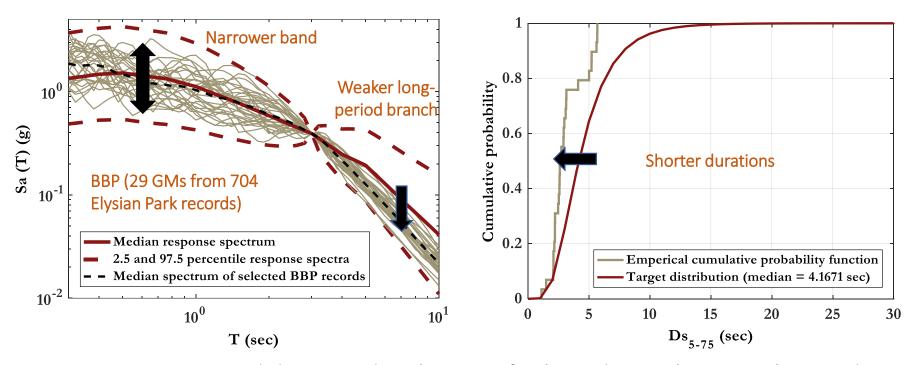
NGA (29 GMs from NGA-WEST)



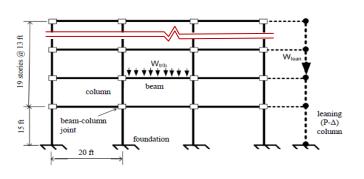
Ground motion selection ($T_{cond} = 3 \text{ s, LADT, } 2\% \text{ in } 200 \text{ years}$)

Discrepancies in selected BBP Elysian Park records at 2% in 200 yr hazard

- 1. steeper descent in the long period range
- **2.** *less variability* in the short period range
- 3. shorter durations

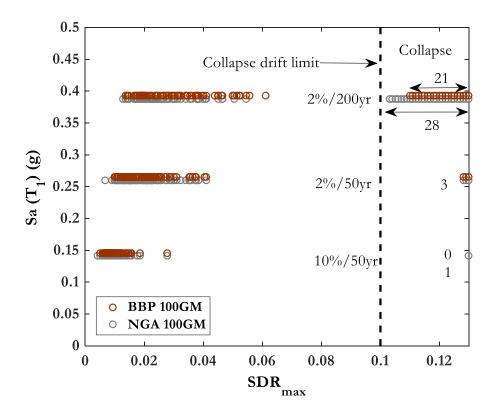


Target vs. Sa and duration distribution of selected BBP Elysian Park records (LADT, 2% in 200yr, $T_{cond} = 3$ sec)



Collapse risk: 20-story frame (LA Downtown)

- NGA and BBP motions yield same number of collapses up to 2% in 50 years hazard
- BBP has: fewer collapses at 2%/200yr: BBP 21 vs. 28 NGA
 - about 10% lower median collapse intensity
 - about **15% lower risk (MAF) of collapse**



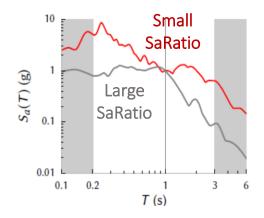
0.5 0.06 **NGA** 0.4 Sa,m = 0.47gProbability of collapse 0.04 0.3 Seismic hazard curve 0.02 BBP $\sqrt{\text{Sa,m}} = 0.52g$ 0.1 0.2 0.3 0.4 0.5 0.1 Sa (g)

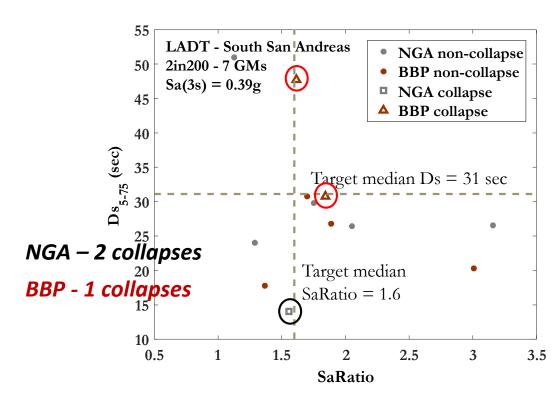
Multi-stripe Stripe Story Drift Response

Collapse Fragility & Hazard Curves

Collapse deaggregation: 20-story frame (LA Downtown)

- Difference at 2%/200yr is checked source by source
- Collapse & non-collapse cases plotted vs. GM Spectral Shape & Duration





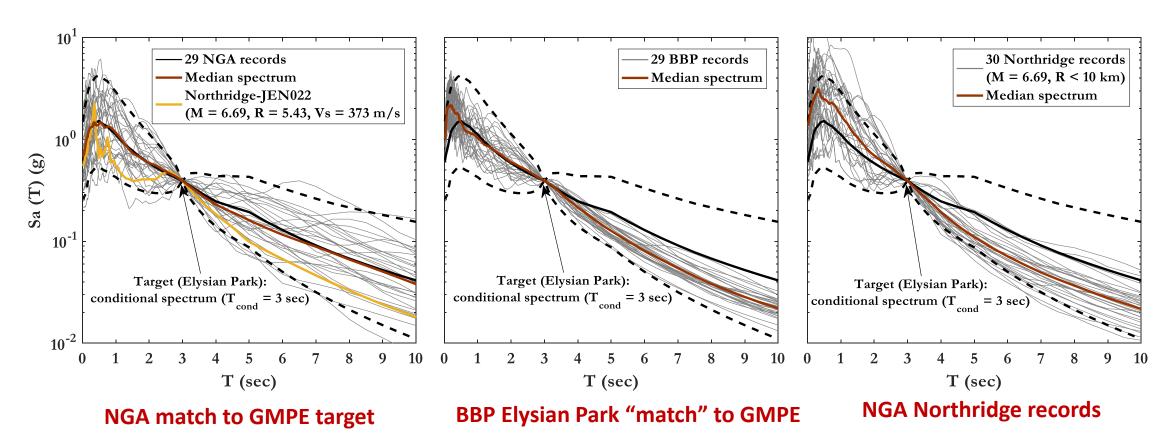
15 LADT - Elysian Park • NGA non-collapse 2in200 - 29 GMs BBP non-collapse Sa(3s) = 0.39g■ NGA collapse ▲ BBP collapse 10 Ds₅₋₇₅ (sec) NGA – 8 collapses BBP - 0 collapses 1.5 2 2.5 SaRatio

Southern San Andreas

Elysian Park

Elysian Park: NGA & BBP records versus Northridge records

- NGA best match: median matches the target spectrum, but the only one Northridge differs
- BBP records: the long-period branch of median spectrum has steeper descent than the target
- Northridge records (w/ M & R): have similar trend with BBP simulation's but more high-frequency content

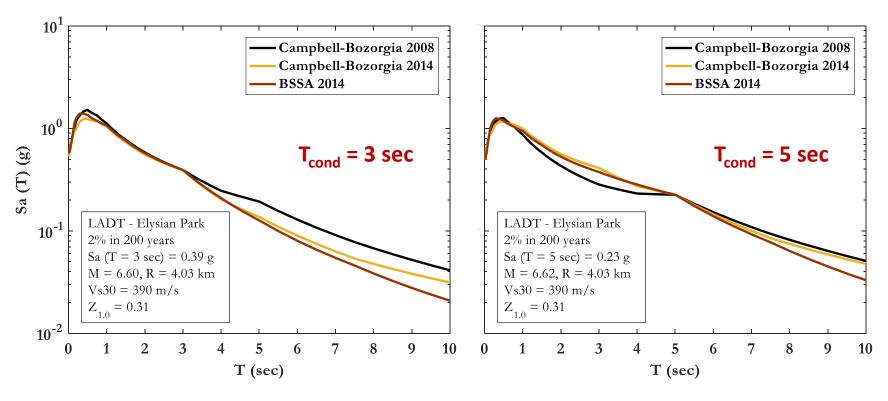


Selected records vs. target spectrum (2%/200yr, CB2008). Seismic hazard deaggregation: Sa = 0.3925 g, R = 4.03 km, M = 6.60, contribution = 29.33%.

Target: CS (GMPE: Campbell-Bozorgnia 2008), Duration (Afshari and Stewart 2016).

Elysian Park: difference in GMPEs?

Seismic hazard deaggregation (for Elysian Park): M = 6.6, R = 4.0 km, Sa = 0.39 g



Conditional Mean Spectra by three GMPEs

Reselect records by 4 schemes to BSSA 2014 target

NGA records

- *M:* 6~8, *R:* 0~175 km
- Vs30: 198~629 m/s
- 14 reverse-fault records (2 Northridge records)
- Maximum scaling: 29

NGA (causal limits)

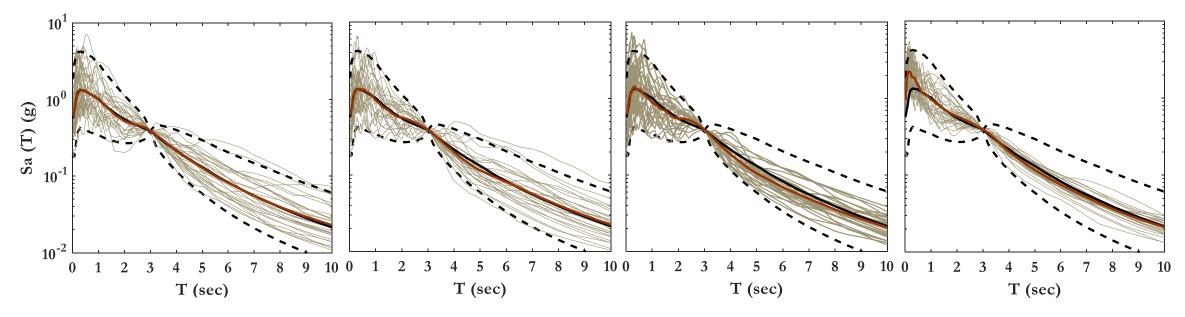
- M: 6~7, R: 0 ~ 10 km
- Vs30: 195 ~ 585 m/s
- 29 reverse-fault records (11 Northridge records)
- Maximum scaling: 23

NGA (causal & scaling limits)

- M: 6~7, R: 0 ~ 10 km
- Vs30: 195 ~ 585 m/s
- 29 reverse-fault records (17 Northridge records)
- *Maximum scaling:* **5**

BBP records

- M = 6.60, R ~ 4.03 km
- Vs30 ~ 390 m/s
- Maximum scaling: 5



Comparison of selected records (T_{cond} = 3 sec) by BSSA-2014 for Elysian Park (2%/200yr).

20-story frame collapse by reselect records by 4 record suites

NGA records

- M: 6~8, R: 0~175 km
- Vs30: 198~629 m/s
- 14 reverse-fault records (2 Northridge records)
- Maximum scaling: **29**

NGA (causal limits)

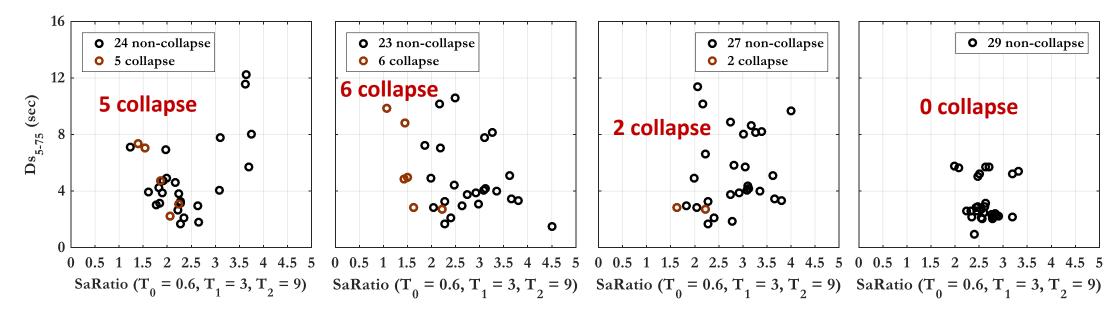
- M: 6~7, R: 0 ~ 10 km
- Vs30: 195 ~ 585 m/s
- 29 reverse-fault records (11 Northridge records)
- *Maximum scaling:* **23**

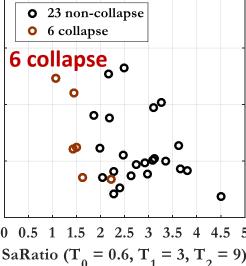
NGA (causal & scaling limits)

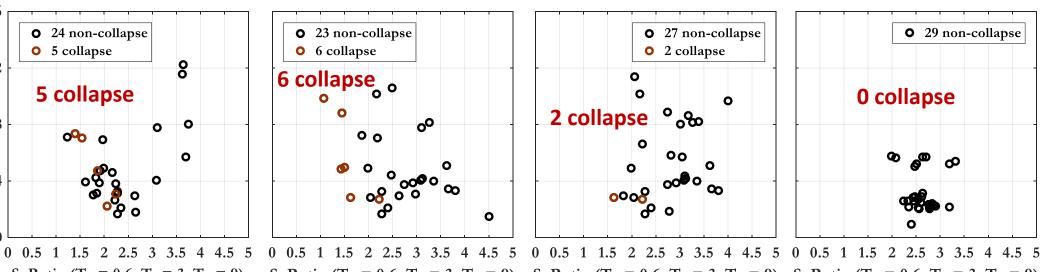
- *M*: 6~7, *R*: 0 ~ 10 km
- Vs30: 195 ~ 585 m/s
- 29 reverse-fault records (17 Northridge records)
- *Maximum scaling:* **5**

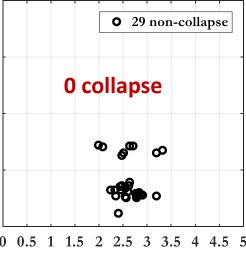
BBP records

- M = 6.60, $R \sim 4.03$ km
- Vs30 ~ 390 m/s
- *Maximum scaling:* **5**







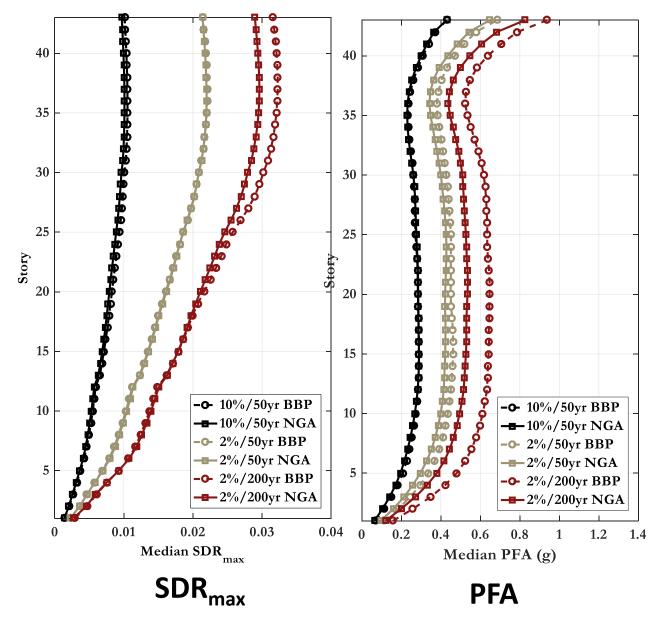


Comparison of building collapses ($T_{cond} = 3 \text{ sec}$, Elysian Park, 2%/200yr).

42-story shear wall building (LADT)

- No statistical difference up to 2% in 50 yr
- ~10% difference under 2% in 200 yr



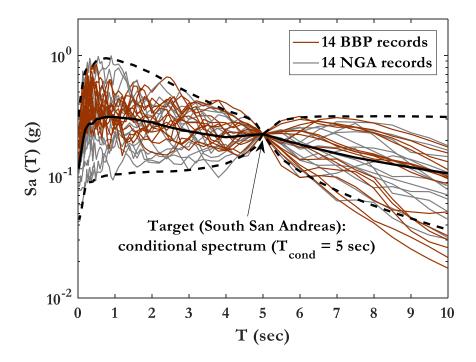


Median structural response demands

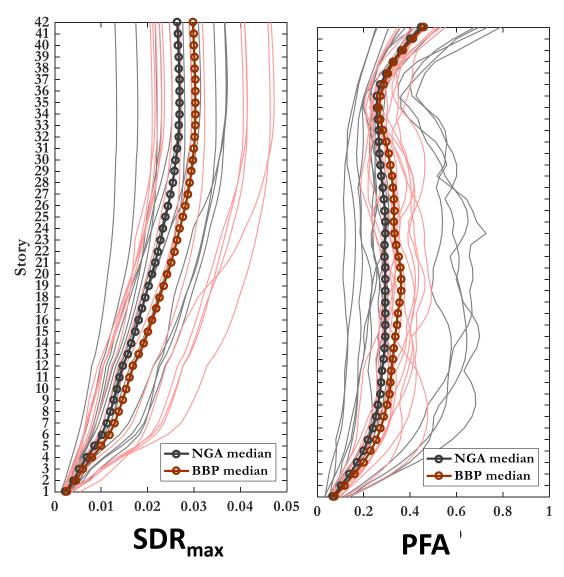
42-story shear wall building (LADT)

San Andreas responses: BBP vs. NGA

- No difference in estimated median responses
- Less dispersion in BBP results



2%/200yr hazard-level South San Andreas motions (14% of LADT seismic hazard)

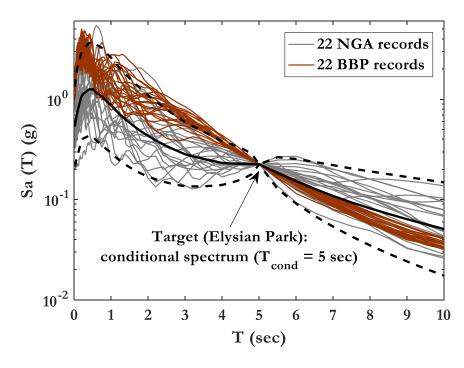


Structural responses under 14 NGA and BBP records

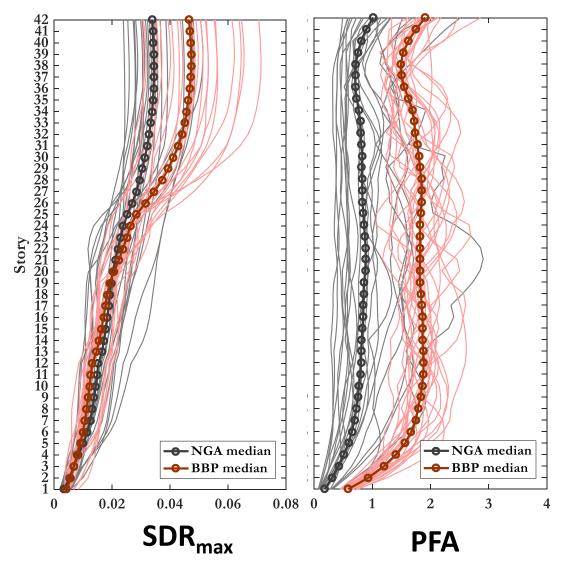
42-story shear wall building (LADT)

Elysian Park responses: BBP vs. NGA

- BBP records -- higher structural demands
- Higher-mode story drifts
- Higher-frequency Sa's and PFA



2%/200yr hazard-level Elysian Park motions (22% of LADT seismic hazard)



Structural responses - 22 NGA and BBP records

Summary (LADT Site)

At 10%/50yr & 2%/50yr level (similar to SF downtown):

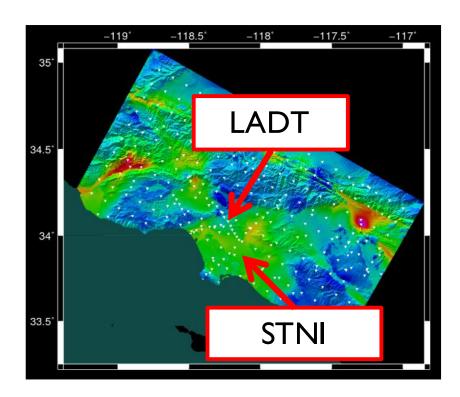
- 1. Simulated (BBP) motions can be selected/scaled to match the PSHA target IMs (CS+D $_{s5-}$ 75) with much smaller scaling than existing recorded motions.
- 2. When ground motions are selected/scaled to comparable target IMs, there are no statistically significant differences in *calculated structural responses and collapse risks* between recorded and simulated motions.

At 2%/200yr level:

- 1. Limitations in our knowledge about rare earthquakes
- 2. The 4 paralleling case studies for Elysian Park scenario indicates that increasing causal and scaling constraints on NGA leads to:
 - more difficulty matching GM's to target spectra
 - selected records tend to be more "local"
 - structural responses tend to converge toward BBP record response
- 3. Importance of high-frequency content for tall buildings: higher-mode drifts, PFA

Engineering Utilization of Cybershake (3D) Motions

Conventional (PSHA w/recorded GMs) vs. CyberShake



Deep Basin Site

LADT: $V_{s30} = 390 \text{ m/s}; Z_{1.0} = 0.3 \text{ km}$

STNI: $V_{s30} = 280 \text{ m/s}$; $Z_{1.0} = 0.9 \text{ km}$

3D Simulations

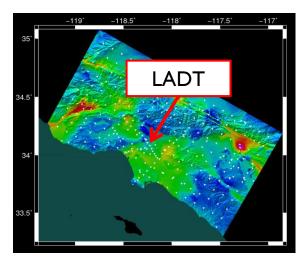
- Velocity Profile (SCEC CVM-S 4.26)
- Wave Propagation

Physics Based & Stochastic (BB)

- Physics Based T > Isecond
- Splicing period ~I second

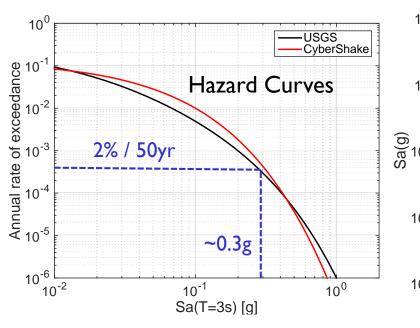
PSHA Approach

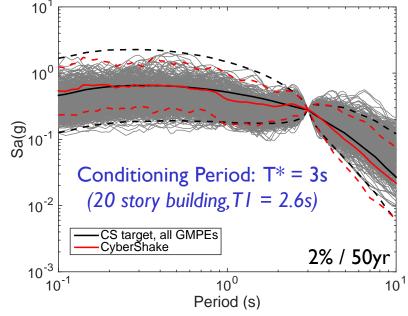
- Multiple ruptures following UCERF 2
- Faults within 200 km

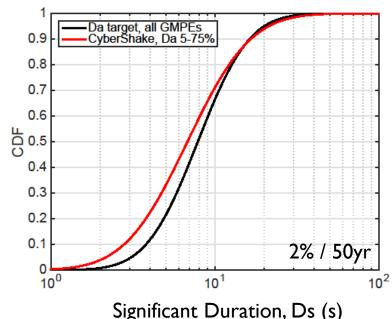


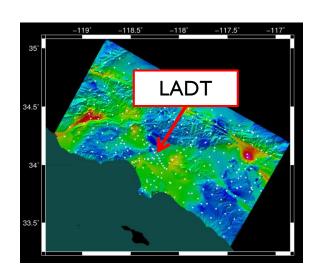
Comparison – CyberShake and Conventional PSHA

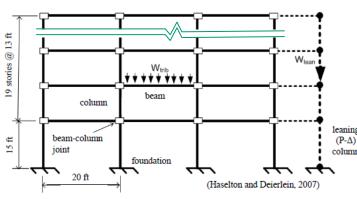
- hazard curve (Sa for T = 3s)
- ground motion CS (Sa, $T^* = 3s$, multiple return periods)
- ground motion Ds (multiple return periods)







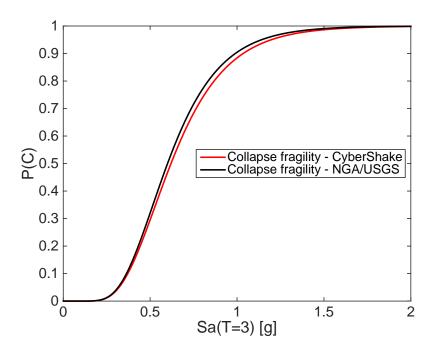




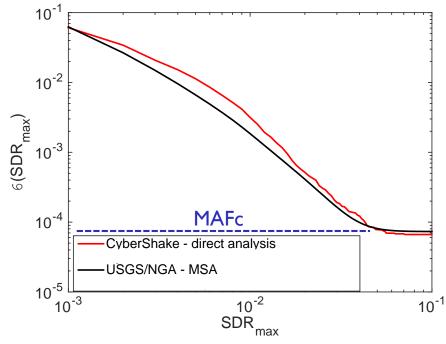
20-story building, $T_1 = 2.60s$

Structural Response Comparisons

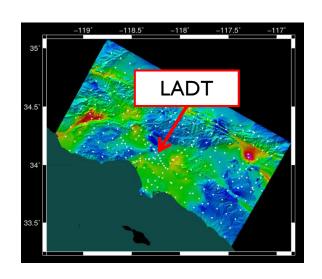
- Collapse conditioned on Sa(T*)
- Story Drift mean annual frequency of exceedence

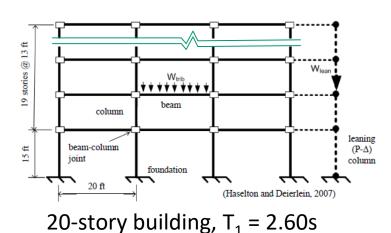






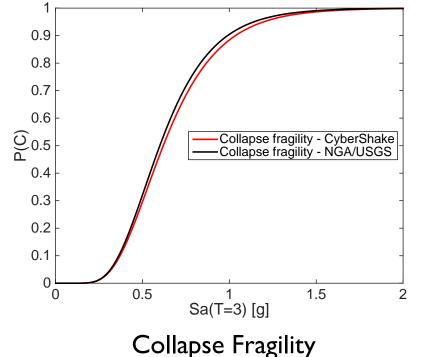
Story Drift Exceedence



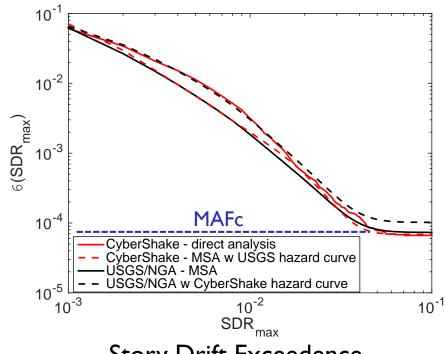


Mix and Match: ground motions with hazard curves

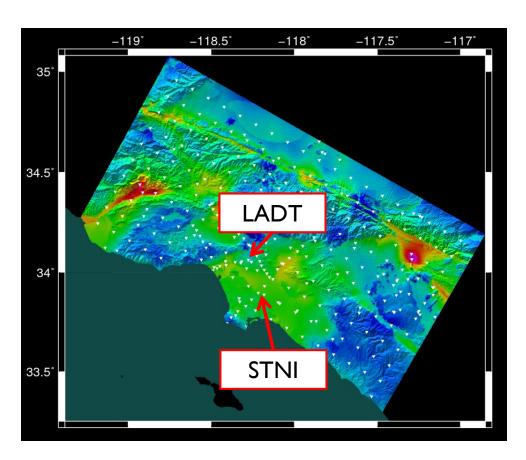
Observation: difference in drift exceedence is attributed primarily to differences in hazard curve







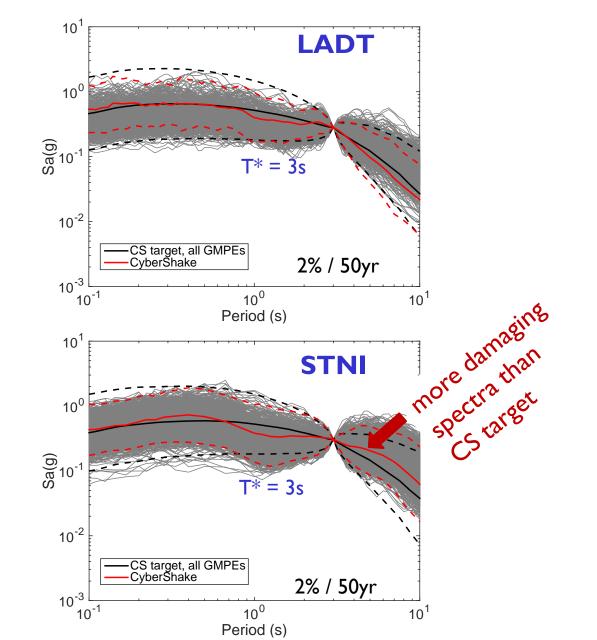
Story Drift Exceedence

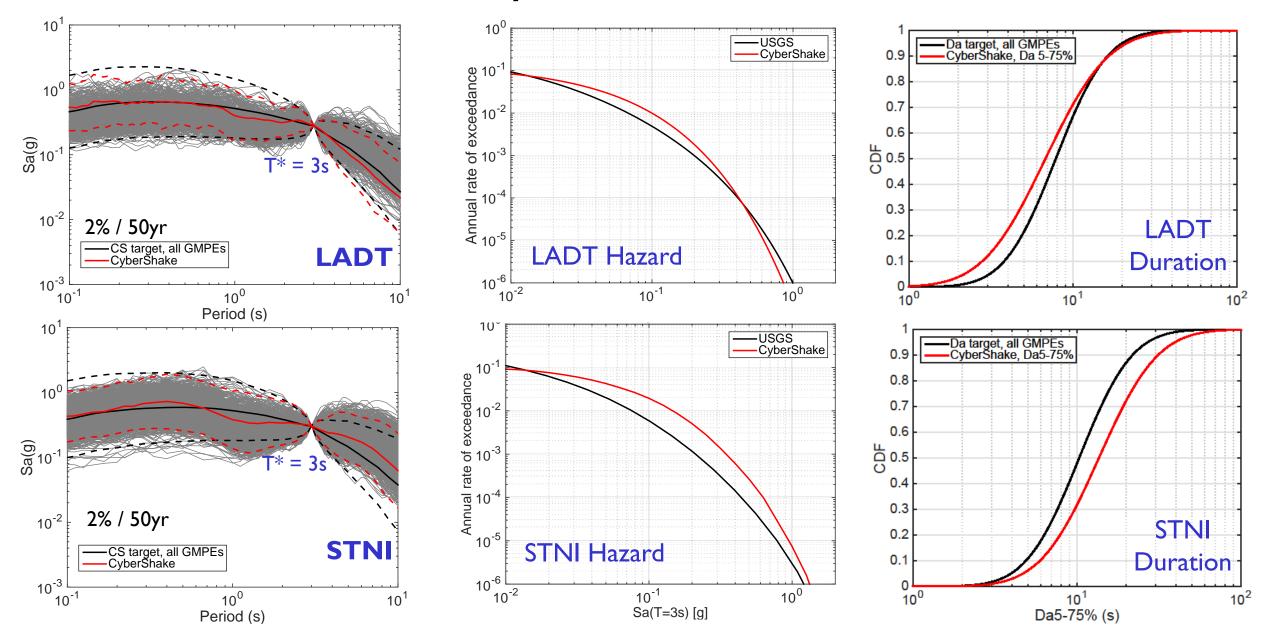


Deep Basin Site

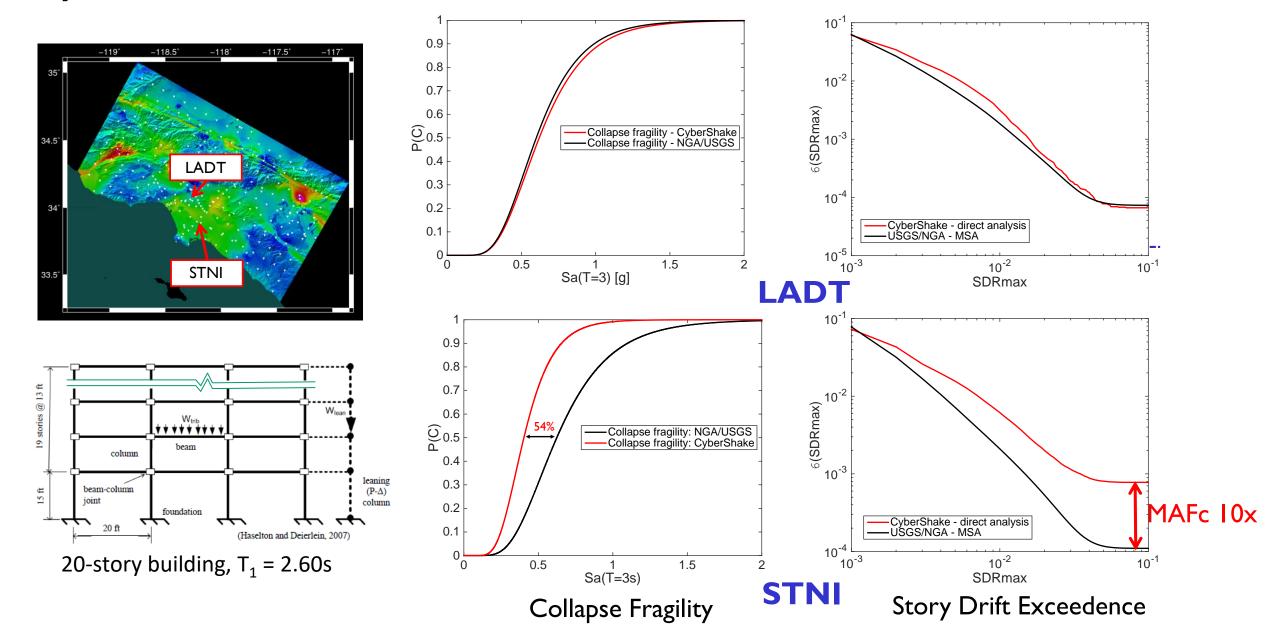
LADT: $V_{s30} = 390 \text{ m/s}$; $Z_{1.0} = 0.3 \text{ km}$

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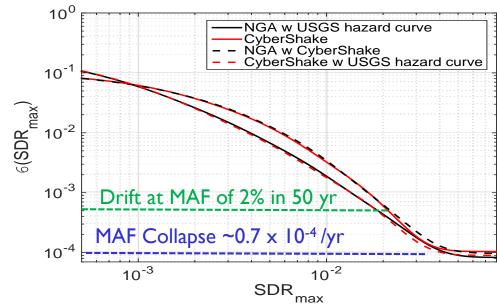


CyberShake: LA downtown vs STNI sites



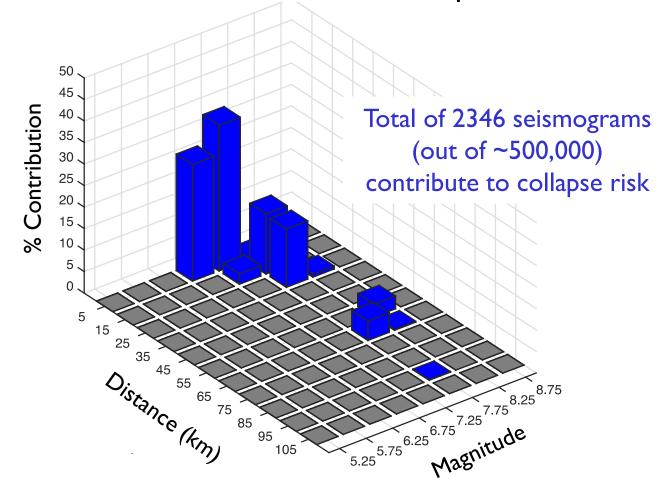
CyberShake: Deaggregation of Risk



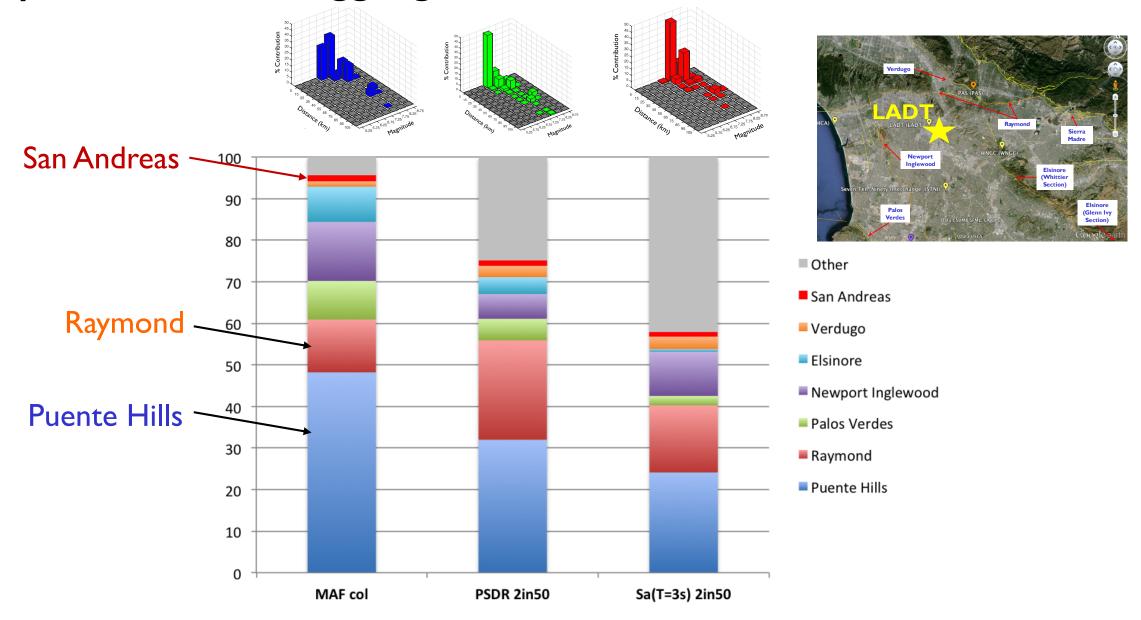


Deaggregation of Risk:

Which earthquake faults and rupture realizations most contribute to collapse?

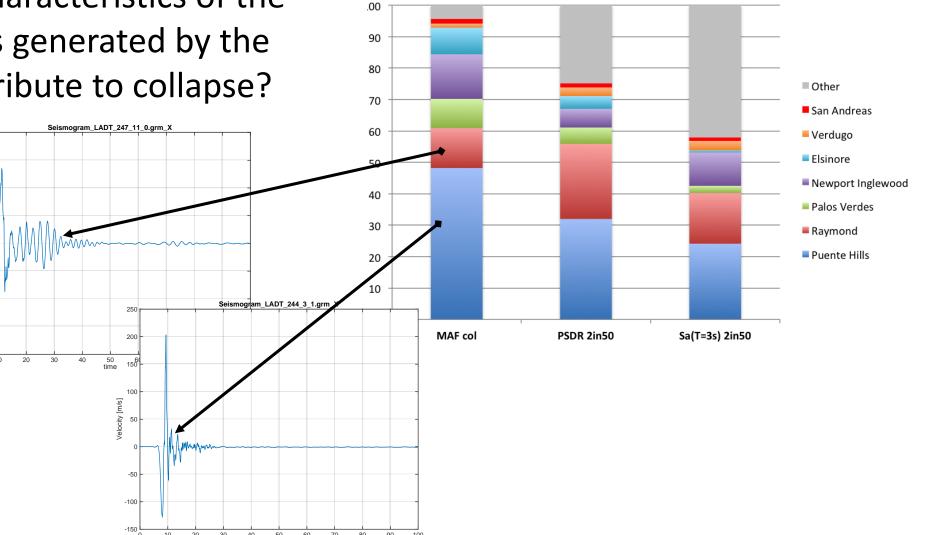


CyberShake: Deaggregation of Risk versus Hazard



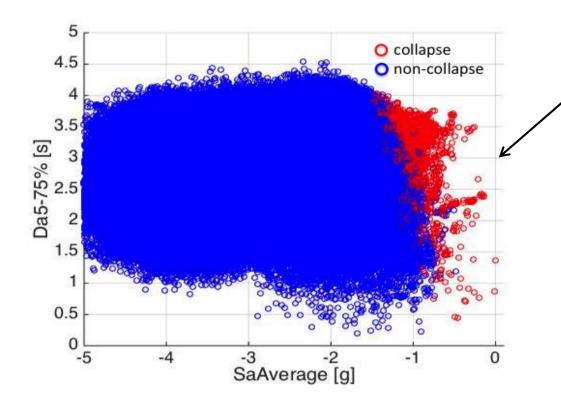
CyberShake: Characterization of Damaging Ground Motions

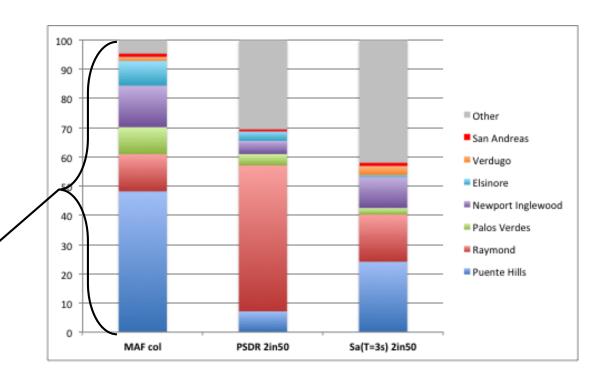
What are the characteristics of the ground motions generated by the faults that contribute to collapse?



CyberShake: Characterization of Damaging Ground Motions

What are the characteristics (intensity measures) of the ground motions that contribute to collapse?





Opportunities to utilize machine learning techniques to interrogate large site specific data sets, identify damaging ground motion characteristics, and relate them to features of the geology and EQ simulation.

Concluding Thoughts

Utilization of Simulated Motions

- IF records are selected and scaled to comprehensive targets (Sa, CS, Ds, correlation/skew), the resulting response is similar between recorded and simulate motions
- BBP motions and GMPE targets match well up to: 2/200yr SF, 2/50yr LADT, 10/50 SB
- Simulated GM's (based on representative events) require less scaling; considered to be more realistic
- ASCE 7 emphasis on causal features (fault type, M,R..) and scaling limits help to capture GM characteristics, BUT limit the number of available GM's

Simulations are most useful where they:

- Provide different answers compared to conventional methods (PSHA w/recorded motions)
- Address situations that are outside the range of conventional methods (large near fault M; basin effects, directivity, etc.).

Continuing studies

- Quantitative validation is important, but can only go so far
- Role of risk deaggregation and sensitivity studies to highlight important features of EQ simulations
- More education and transparency in confidence in models and assumptions